***8–1.** Determine the equations of the elastic curve for the beam using the x_1 and x_2 coordinates. Specify the slope at *A* and the maximum deflection. *EI* is constant.



 $EIv_1 = \frac{Px_1^2}{6}C_1x_1 + C_1$

 $EI\frac{d^2v}{dx^2} = M(x)$

 $EI\frac{d^2v_1}{dx_1^2} = Px_1$

 $EI\frac{dv_1}{dx_1} = \frac{Px_1^2}{2} + C_1$

For $M_1(x) = Px_1$

For $M_1(x) = Pa$

$$EI\frac{d^2v_1}{dx_1^2} = Pa$$
$$EI\frac{dv_1}{dx_1} = Pax_1 + C_1$$

$$EIv_1 = \frac{Pax_1^2}{2} = C_3 x_1 + C_4 \tag{4}$$

Boundary conditions:

 $v_1 = 0$ at x = 0

From Eq. (2)

$$C_2 = 0$$

Due to symmetry:

$$\frac{dv_1}{dx_1} = 0 \quad \text{at} \quad x_1 = \frac{L}{2}$$

From Eq. (3)

$$0 = Pa\frac{L}{2} + C_3$$
$$C_3 = \frac{PaL}{2}$$

Continuity conditions:

$$v_{1} = v_{2} \text{ at } x_{1} = x_{2} = a$$

$$\frac{Pa^{3}}{6} + C_{1}a = \frac{Pa^{3}}{2} - \frac{Pa^{3}L}{2} + C_{4}$$

$$C_{1}a \cdot C_{4} = \frac{Pa^{3}}{2} - \frac{Pa^{3}L}{2}$$

$$\frac{dv_{1}}{dx_{1}} = \frac{dv_{2}}{dx_{2}} \text{ at } x_{1} = x_{2} = a$$
(5)

Ans.

8–1. Continued

$$\frac{Pa^3}{2} + C_1 = Pa^3 - \frac{PaL}{2}$$
$$C_1 = \frac{Pa^2}{2} - \frac{PaL}{2}$$

Substitute C_1 into Eq. (5)

$$C_{a} = \frac{Pa^{3}}{6}$$

$$\frac{dv_{1}}{dx_{1}} = \frac{P}{2EI}(x_{1}^{2} + a^{2} - aL)$$

$$\theta_{A} = \frac{dv_{1}}{dx_{1}}\Big|_{x_{1}=0} = \frac{Pa(a - L)}{2EI}$$

$$w_{1} = \frac{Px_{1}}{6EI}[x_{1}^{2} + 3a(a - L)]$$

$$w_{2} = \frac{Pa}{6EI} + (3x_{2}(x_{2} - L) + a^{2})$$
Ans

$$V_{x=1} = V_2 \bigg|_{x=\frac{1}{2}} = \frac{Pa}{24EI} (4a^2 - 3L^2)$$
 Ans.

8–2. The bar is supported by a roller constraint at *B*, which allows vertical displacement but resists axial load and moment. If the bar is subjected to the loading shown, determine the slope at A and the deflection at C. EI is constant.

$$EI\frac{d^{2}v_{1}}{dx_{1}^{2}} = M_{1} = Px_{1}$$

$$EI\frac{dv_{1}}{dx_{2}} = \frac{Px_{1}^{2}}{2} + C_{1}$$

$$EIv_{1} = \frac{Px_{1}^{2}}{6} + C_{1}x_{1} + C_{1}$$

$$EI\frac{d^{2}v_{2}}{dx_{2}} = M_{2} = \frac{PL}{2}$$

$$EI\frac{dv_{2}}{dx_{2}} = \frac{PL}{2}x_{2} + C_{3}$$

$$EIv_{2} = \frac{PL}{4}x_{2}^{2} + C_{3}x_{3} + C_{4}$$



8–2. Continued

Boundary conditions:

At
$$x_1 = 0$$
, $v_1 = 0$
 $0 = 0 + 0 + C_2$; $C_2 = 0$
At $x_2 = 0$, $\frac{dv_2}{dx_2} = 0$
 $0 + C_3 = 0$; $C_3 = 0$
At $x_1 = \frac{L}{2}$, $x_2 = \frac{L}{2}$, $v_1 = v_2$, $\frac{dv_1}{dx_1} = -\frac{dv_2}{dx_2}$
 $\frac{P\left(\frac{L}{2}\right)^2}{6} + C_1\left(\frac{L}{2}\right) = \frac{PL\left(\frac{L}{2}\right)^2}{4} + C_4$
 $\frac{P\left(\frac{L}{2}\right)^2}{2} + C_1 = -\frac{P\left(\frac{L}{2}\right)}{2}$; $C_1 = -\frac{3}{8}PL^3$
 $C_4 = -\frac{11}{48}PL^3$
At $x_1 = 0$
 $\frac{dv_1}{dx_1} = \theta_A = -\frac{3}{8}\frac{PL^2}{EI}$
At $x_1 = \frac{L}{2}$
 $v_c = \frac{P\left(\frac{L}{2}\right)^3}{6EI} - \left(\frac{3}{8}PL^2\right)\left(\frac{L}{2}\right) + 0$
 $v_c = -\frac{PL^3}{6EI}$



Ans.

Ans.

8–3. Determine the deflection at *B* of the bar in Prob. 8–2.

$$EI\frac{d^{2}v_{1}}{dx_{1}^{2}} = M_{1} = Px_{1}$$

$$EI\frac{dv_{2}}{dx_{1}} = \frac{Px_{1}^{2}}{2} + C_{1}$$

$$EIv_{1} = \frac{Px_{1}^{2}}{6} + C_{1}x_{1} + C_{2}$$

$$EI\frac{d^{2}v_{2}}{dx_{2}} = M_{2} = \frac{PL}{2}$$

$$EI\frac{dv_{2}}{dx_{2}}\frac{PL}{2}x_{2} + C_{3}$$

$$EIv_{2} = \frac{PL}{4}x_{2}^{2} + C_{3}x_{2} + C_{4}$$



8–3. Continued

Boundary conditions:
At $x_1 = 0$, $v_1 = 0$
$0 = 0 + 0 + C_2; C_2 = 0$
At $x_2 = 0$, $\frac{dv_2}{dx_2} = 0$
$0 + C_3 = 0; C_3 = 0$
At $x_1 = \frac{L}{2}$, $x_2 = \frac{L}{2}$, $v_1 = v_2$, $\frac{dv_1}{dx_1} = -\frac{dv_2}{dx_2}$
$\frac{P\left(\frac{L}{2}\right)^3}{6} + C_1\left(\frac{L}{2}\right) = \frac{PL\left(\frac{L}{2}\right)^2}{4} + C_4$
$\frac{P\left(\frac{L}{2}\right)^{3}}{2} + C_{1} = -\frac{P\left(\frac{L}{2}\right)}{2}; C_{1} = -\frac{3}{8}PL^{2}$
$C_4 = \frac{11}{48} P L^3$
$At x_2 = 0,$
$v_B = -\frac{11PL^3}{48EI}$

*8–4. Determine the equations of the elastic curve using the coordinates x_1 and x_2 , specify the slope and deflection at *B*. *EI* is constant.

$$EI\frac{d^2v}{dx^2} = M(x)$$

For
$$M_1(x) = -\frac{w}{2}x_1^2 + wax_1 - \frac{wa^2}{2}$$

 $EI\frac{d^2v_1}{dx_1^2} = -\frac{w}{2}x_1^2 + wax_1 - \frac{wa^2}{2}$
 $EI\frac{dv_1}{dx_1} = -\frac{w}{6}x_1^3 + \frac{wa}{2}x_1^2 - \frac{wa^2}{2}x_1 + C_1$



w

Ans.

(1)

8-4. Continued

$$EIv_{1} = -\frac{w}{24}x_{1}^{4} + \frac{wa}{6}x_{1}^{3} - \frac{wa^{2}}{4}x_{1}^{2} + C_{1}x_{1} + C_{2} \qquad (2)$$
For $M_{2}(x) = 0; EI\frac{d^{2}v_{2}}{dx_{2}^{3}} = 0$

$$EI\frac{dv_{2}}{dx_{2}} = C_{3} \qquad (3)$$

$$EIv_{2} = C_{3}x_{2} + C_{4} \qquad (4)$$
Boundary conditions:
At $x_{1} = 0, \frac{dv_{1}}{dx_{1}} = 0$
From Eq. (1), $C_{1} = 0$
At $x_{1} = 0, v_{1} = 0$
From Eq. (2): $C_{2} = 0$
Continuity conditions:
At $x_{1} = a, x_{2} = a; \frac{dv_{1}}{dx_{1}} = \frac{dv_{2}}{dx_{2}}$
From Eqs. (1) and (3),
 $-\frac{wa^{3}}{6} + \frac{wa^{3}}{2} - \frac{wa^{3}}{2} = C_{3}; C_{3} = -\frac{wa^{3}}{6}$
From Eqs. (2) and (4),
At $x_{1} = a, x_{2} = a v_{1} = v_{2}$
 $-\frac{wa^{4}}{24} + \frac{wa^{4}}{6} - \frac{wa^{4}}{4} = -\frac{wa^{4}}{6} + C_{4}; C_{4} = \frac{wa^{4}}{24}$
The slope, from Eq. (3),

$$\theta_B = \frac{dv_2}{dx_2} = \frac{wa^3}{6EI}$$

The elastic curve:

$$v_{1} = \frac{w}{24EI} \left(-x_{1}^{4} + 4ax_{1}^{3} - 6a^{2}x_{1}^{2} \right)$$

$$v_{2} = \frac{wa^{3}}{24EI} \left(-4x_{2} + a \right)$$

$$v_{1} = v_{2} \bigg|_{x_{3} = L} = \frac{wa^{3}}{24EI} \left(-4L + a \right)$$
Ans

wax - waz M.(x) =

Wa

a/2 a/2

waz

Wa



IS.

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8–5. Determine the equations of the elastic curve using the coordinates x_1 and x_3 , and specify the slope and deflection at point *B*. *EI* is constant.

$$EI\frac{d^{2}v}{dx_{2}} = M(x)$$

For $M_{1}(x) = -\frac{w}{2}x_{1}^{2} + wax_{1} - \frac{wa^{2}}{2}$
 $EI\frac{d^{2}v_{1}}{dx_{1}^{2}} = -\frac{w}{2}x_{1}^{2} + wax_{1} - \frac{wa^{2}}{2}$
 $EI\frac{dv_{1}}{dx_{1}} = -\frac{w}{6}x_{1}^{3} + \frac{wa}{2}x_{1}^{2} - \frac{wa^{2}}{2}x_{1} + C_{1}$
 $EIv_{1} = -\frac{w}{24}x_{1}^{4} + \frac{wa}{6}x_{1}^{3} - \frac{wa^{2}}{4}x_{1}^{2} + C_{1}x_{1} + C_{2}$
For $M_{2}(x) = 0; EI\frac{d^{2}v_{3}}{dx_{3}^{2}} = 0$
 $EI\frac{dv_{3}}{dx_{3}} = C_{3}$
 $EIv_{3} = C_{3}x_{3} + C_{4}$

Boundary conditions:

At $x_1 = 0$, $\frac{dv_1}{dx_1} = 0$

From Eq. (1),

 $0 = -0 + 0 - 0 + C_1; \quad C_1 = 0$ At $x_1 = 0, \quad v_1 = 0$

From Eq. (2),

$$0 = -0 - 0 - 0 + 0 + C_2; \quad C_2 = 0$$

Continuity conditions:

At
$$x_1 = a$$
, $x_3 = L - a$; $\frac{dv_1}{dx_1} = \frac{dv_3}{dx_3}$
 $-\frac{wa^3}{6} + \frac{wa^3}{2} - \frac{-wa^3}{2} = -C_3$; $C_3 = +\frac{wa^3}{6}$
At $x_1 = a$, $x_3 = L - a$ $v_1 = v_2$
 $-\frac{wa^4}{24} + \frac{wa^4}{6} - \frac{wa^4}{4} = \frac{wa^3}{6}(L - a) + C_4$; $C_4 = \frac{wa^4}{24} - \frac{wa^3L}{6}$

The slope

$$\frac{dv_3}{dx_3} = \frac{wa^3}{6EI}$$

$$\theta_B = \frac{d_{v3}}{d_{x3}} \bigg|_{x_3=0} = \frac{wa^3}{6EI}$$
Ans.

The elastic curve:

$$v_{1} = \frac{wx_{1}^{2}}{24EI} \left(-x_{1}^{2} + 4ax_{1} - 6a^{2} \right)$$

$$v_{3} = \frac{wa^{3}}{24EI} \left(4x_{3} + a - 4L \right)$$

$$V_{2} = V_{3} \bigg|_{x_{3} = 0} = \frac{wa^{3}}{24EI} \left(a - 4L \right)$$
Ans.
Ans.



С

8–6. Determine the maximum deflection between the supports A and B. EI is constant. Use the method of integration.

Elastic curve and slope:

$$EI\frac{d^{2}v}{dx^{2}} = M(x)$$

For $M_{1}(x) = \frac{-wx_{1}^{2}}{2}$
 $EI\frac{d^{1}v_{1}}{dx_{1}^{2}} = \frac{-wx_{1}^{2}}{2}$
 $EI\frac{dv_{1}}{dx_{1}} = \frac{-wx_{1}^{3}}{6} + C_{1}$
 $EIv_{1} = -\frac{wx_{1}^{4}}{24} + C_{1}x_{1} + C_{2}$

For
$$M_2(x) = \frac{-wLx_2}{2}$$

$$EI\frac{d^{2}v_{2}}{dx_{3}^{2}} = \frac{-wLx_{2}}{2}$$
$$EI\frac{dv_{2}}{dx_{2}} = \frac{-wLx_{2}^{2}}{4} + C_{3}$$

$$EIv_2 = \frac{-wLx_2^2}{412} + C_3x_3 + C_4 \tag{4}$$

Boundary conditions:

$$v_{2} = 0 \text{ at } x_{2} = 0$$

From Eq. (4):
$$C_{4} = 0$$

$$v_{2} = 0 \text{ at } x_{2} = L$$

From Eq. (4):
$$0 = \frac{-wL^{4}}{12} + C_{3}L$$

$$C_{3} = \frac{wL^{3}}{12}$$

$$v_{1} = 0 \text{ at } x_{1} = L$$

From Eq. (2):

$$0 = -\frac{wL^4}{24} + C_1L + C_2$$



(5)

8–6. Continued

Continuity conditions:

$$\frac{dv_1}{dx_1} = \frac{dv_2}{-dx_2} \text{ at } x_1 = x_2 = L$$

From Eqs. (1) and (3)
$$-\frac{wL^3}{6} + C_1 = -\left(-\frac{wL^3}{4} + \frac{wL^3}{12}\right)$$

$$C_1 = \frac{wL^3}{3}$$

Substitute C₁ into Eq. (5)
$$C_2 = \frac{7wL^4}{24}$$

$$\frac{dv_1}{dx_1} = \frac{w}{6EI}(2L^3 - x_1^3)$$

$$\frac{dv_2}{dx_2} = \frac{w}{12EI}(L^3 - 3Lx_2^2)$$

$$\theta_A = \frac{d_{v1}}{d_{x1}} \bigg|_{x_1 = L} = -\frac{dv_2}{dv_3} \bigg|_{x_3 = L} = \frac{wL^3}{6EI}$$

$$v_1 = \frac{w}{24EI}(-x_1^4 + 8L^3x_1 - 7L^4)$$

$$(v_1)_{\max} = \frac{-7wL^4}{24EI}(x_1 = 0)$$

The negative sign indicates downward displacement

$$v_{2} = \frac{wL}{12EI}(L^{2}x_{2} - x_{2}^{3})$$

$$(v_{2})_{\text{max}} \text{ occurs when } \frac{dv_{2}}{dx_{2}} = 0$$
From Eq. (6)
$$(7)$$

 $L^3 - 3Lx_2^2 = 0$

$$x_2 = \frac{L}{\sqrt{3}}$$

Substitute x_2 into Eq. (7),

$$(v_2)_{\max} = \frac{wL_4}{18\sqrt{3EI}}$$

Ans.

(6)

8–7. Determine the elastic curve for the simply supported beam using the x coordinate $0 \le x \le L/2$. Also, determine the slope at A and the maximum deflection of the beam. EI is constant.

 $EI\frac{d^2v}{dx^2} = M(x)$

At x = 0, v = 0From Eq. (2),

From Eq. (1),

From Eq. (2),



Α

*8-8. Determine the equations of the elastic curve using the coordinates x_1 and x_2 , and specify the slope at *C* and displacement at *B*. *EI* is constant.

Support Reactions and Elastic Curve: As shown on FBD(a). Moment Function: As shown on FBD(c) and (c). Slope and Elastic Curve:

$$EI\frac{d^2v}{dx^2} = M(x)$$

For $M(x_1) = wax_1 - \frac{3wa^2}{2}$,

$$EI\frac{d^2v_1}{dx_1^2} = wax_1 - \frac{3wa^2}{2}$$
$$EI\frac{dv_1}{dx_1} = \frac{wa}{2}x_1^2 - \frac{3wa^2}{2}x_1 + C_1$$
$$EIv_1 = \frac{wa}{6}x_1^3 - \frac{3wa^2}{4}x_1^2 + C_1x_1 + C_2$$

For
$$M(x_2) = -\frac{w}{2}x_2^2$$

$$EI\frac{d^{2}v_{2}}{dx_{2}^{2}} = -\frac{w}{2}x_{2}^{2}$$

$$EI\frac{dv_{2}}{dx_{2}} = -\frac{w}{6}x_{2}^{3} + C_{3}$$

$$EIv_{2} = \frac{w}{24}x_{2}^{4} + C_{3}x_{2} + C_{4}$$
(4)

Boundary Conditions:

 $\frac{dv_1}{dx_1} = 0 \text{ at } x_1 = 0, \quad \text{From Eq. [1]}, \quad C_1 = 0$ $v_1 = 0 \text{ at } x_1 = 0 \quad \text{From Eq. [2]}, \quad C_2 = 0$

Continuity Conditions:

At
$$x_1 = a$$
 and $x_2 = a$, $\frac{dv_1}{dx_1} = -\frac{dv_2}{dx_2}$ From Eqs. [1] and [3],
 $\frac{wa^3}{2} - \frac{3wa^3}{2} = -\left(-\frac{wa^3}{6} + C_3\right)$ $C_3 = \frac{7wa^3}{6}$
At $x_1 = a$ and $x_2 = a$, $v_1 = v_2$. From Eqs. [2] and [4],
 $\frac{wa^4}{6} - \frac{3wa^4}{4} = -\frac{wa^4}{24} + \frac{5wa^4}{6} + C_4$ $C_4 = -\frac{41wa^4}{8}$

The Slope: Substituting into Eq. [1],

$$\frac{dv_1}{dx_1} = \frac{wax_1}{2EI}(x_1 - 3a)$$

$$\theta_C = \frac{dv_2}{dx_2} \bigg|_{x_1 = a} = -\frac{wa^3}{EI}$$
Ans.

The Elastic Curve: Substituting the values of C_1 , C_2 , C_3 , and C_4 into Eqs. [2] and [4], respectively

$$v_{1} = \frac{wax_{1}}{12EI}(2x_{1}^{2} - 9ax_{1})$$

$$v_{2} = \frac{w}{24EI}(-x_{2}^{4} + 28a^{3}x_{2} - 41a^{4})$$

$$v_{B} = v_{2}\Big|_{x_{2}=0} = -\frac{41wa^{4}}{24EI}$$
Ans.

$$M(X_{a}) = -\frac{WX_{a}}{2}$$

$$V(X_{a}) = \frac{WX_{a}}{2}$$

$$V(X_{a}) = \frac{WX_{a}}{2}$$

A

(1)

(2)

8-9. Determine the equations of the elastic curve using the coordinates x_1 and x_3 , and specify the slope at B and deflection at C. EI is constant.

Support Reactions and Elastic Curve: As shown on FBD(a). Moment Function: As shown on FBD(b) and (c). Slope and Elastic Curve:

$$EI\frac{d^2v}{dx^2} = M(x)$$

For $M(x_1) = wax_1 - \frac{3wa^2}{2}$,
$$EI\frac{d^2v_1}{dx_1^2} = wax_1 - \frac{3wa^2}{2}$$
$$EI\frac{dv_1}{dx_1} = \frac{wa}{2}x_1^2 - \frac{3wa^2}{2}x_1 + C_1$$
$$EIv_1 = \frac{wa}{6}x_1^3 - \frac{3wa^2}{4}x_1^2 + C_1x_1 + C_2$$

For
$$M(x_3) = 2wax_3 - \frac{w}{2}x_2^3 - 2wa^2$$
,
 $EI\frac{d^2v_3}{dx_3^2} = 2wax_3 - \frac{w}{2}x_3^2 - 2wa^2$
 $EI\frac{dv_3}{dx_3} = wax_3^2 - \frac{w}{6}x_3^3 - 2wa^2x_3 + C_3$ (3)
 $EIv_3 = \frac{wa}{3}x_3^3 - \frac{w}{24}x_3^4 - wa^2x_3^2 + C_3x_3 + C_4$ (4)

а

Boundary Conditions:

 $\frac{dv_1}{dx_1} = 0$ at $x_1 = 0$, From Eq. [1], $C_1 = 0$ From Eq. [2], $C_2 = 0$ $v_1 = 0$ at $x_1 = 0$,

Continuity Conditions:

6

At
$$x_1 = a$$
 and $x_3 = a$, $\frac{dv_1}{dx_1} = \frac{dv_3}{dx_3}$ From Eqs. [1] and [3],
 $\frac{wa^3}{2} - \frac{3wa^3}{2} = wa^3 - \frac{wa^3}{6} - 2wa^3 + C_3$ $C_3 = \frac{wa^3}{6}$
At $x_1 = a$ and $x_3 = a$, $v_1 = v_3$, From Eqs.[2] and [4],
 $\frac{wa^4}{6} - \frac{3wa^4}{4} = \frac{wa^4}{3} - \frac{wa^4}{24} - wa^4 + \frac{wa^4}{6} + C_4$ $C_4 = -\frac{wa^4}{24}$

The Slope: Substituting the value of C₃ into Eq. [3],

4

$$\frac{dv_3}{dx_3} = \frac{w}{2EI}(6ax_3^2 - x_3^3 - 12a^2x_3 + a^3)$$

$$\theta_B = \frac{dv_3}{dx_3}\Big|_{x_3 = 2a} = -\frac{7wa^3}{6EI}$$
Ans.

The Elastic Curve: Substituting the values of C_1, C_2, C_3 , and C_4 into Eqs. [2] and [4], respectively,

$$v_1 = \frac{wax_1}{12EI}(2x_1^2 - 9ax_1)$$
 Ans.

$$v_C = v_1 \Big|_{x_1 = a} = -\frac{7wa^4}{12EI}$$
 Ans.

$$v_3 = \frac{w}{24EI}(-x_3^4 + 8ax_3^3 - 24a^2x_3^2 + 4a^3x_3 - a^4)$$
 Ans.

8–10. Determine the slope at *B* and the maximum displacement of the beam. Use the moment-area theorems. Take $E = 29(10^3)$ ksi, I = 500 in⁴.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\theta_{B} = |\theta_{B/A}| = \frac{1}{2} \left(\frac{90 \text{ k} \cdot \text{ft}}{EI}\right) (6 \text{ ft})$$

$$= \frac{270 \text{ k} \cdot \text{ft}^{2}}{EI} = \frac{270 (144) \text{ k} \cdot \text{in}^{2}}{\left[29(10^{3})\frac{\text{k}}{\text{in}^{2}}\right] (500 \text{ in}^{4})} = 0.00268 \text{ rad} \quad \forall \qquad \text{Ans.}$$

$$\Delta_{\text{max}} = \Delta_{\text{C}} = |\textbf{t}_{\text{B/A}}| = \left[\frac{1}{2} \left(\frac{90 \text{ k} \cdot \text{ft}}{EI}\right) (6 \text{ ft})\right] \left[6 \text{ ft} + \frac{2}{3} (6 \text{ ft})\right]$$

$$= \frac{2700 \text{ k} \cdot \text{ft}^{3}}{EI}$$

$$= \frac{2700 (1728) \text{ k} \cdot \text{in}^{3}}{\left[29(10^{3})\frac{\text{k}}{\text{in}^{2}}\right] (500 \text{ in}^{4})}$$

$$= 0.322 \text{ in } \downarrow \qquad \text{Ans.}$$

***8–12.** Determine the slope and displacement at *C*. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\begin{aligned} \theta_{C/A} &= \frac{1}{2} \left(-\frac{225 \text{ k} \cdot \text{ft}}{EI} \right) (45 \text{ ft}) = -\frac{5062.5 \text{ k} \cdot \text{ft}^2}{EI} = \frac{5062.5 \text{ k} \cdot \text{ft}^2}{EI} \quad \forall \\ |t_{B/A}| &= \left[\frac{1}{2} \left(\frac{225 \text{ k} \cdot \text{ft}}{EI} \right) (30 \text{ ft}) \right] \left[\frac{1}{3} (30 \text{ ft}) \right] = \frac{33750 \text{ k} \cdot \text{ft}^3}{EI} \\ |t_{C/A}| &= \left[\frac{1}{2} \left(\frac{225 \text{ k} \cdot \text{ft}}{EI} \right) (30 \text{ ft}) \right] \left[15 \text{ ft} + \frac{1}{3} (30 \text{ ft}) \right] + \left[\frac{1}{2} \left(\frac{225 \text{ k} \cdot \text{ft}}{EI} \right) (15 \text{ ft}) \right] \left[\frac{2}{3} (15 \text{ ft}) \\ &= \frac{101250 \text{ k} \cdot \text{ft}^3}{EI} \end{aligned}$$

Then,

$$\Delta' = \frac{45}{30}(t_{B/A}) = \frac{45}{30}\left(\frac{33750 \text{ k} \cdot \text{ft}^3}{EI}\right) = \frac{50625 \text{ k} \cdot \text{ft}^3}{EI}$$
$$\theta_A = \frac{|t_{B/A}|}{L_{AB}} = \frac{33750 \text{ k} \cdot \text{ft}^3/EI}{30 \text{ ft}} = \frac{1125 \text{ k} \cdot \text{ft}^2}{EI} \checkmark$$
$$+ \mathcal{P} \theta_C = \theta_A + \theta_{C/A}$$
$$\theta_C = \frac{-1125 \text{ k} \cdot \text{ft}^2}{EI} + \frac{5062.5 \text{ k} \cdot \text{ft}^2}{EI} = \frac{3937.5 \text{ k} \cdot \text{ft}^2}{EI} \checkmark$$
$$\Delta_C = \left|t_{C/A}\right| - \Delta' = \frac{101250 \text{ k} \cdot \text{ft}^3}{EI} - \frac{50625 \text{ k} \cdot \text{ft}^3}{EI}$$
$$= \frac{50625 \text{ k} \cdot \text{ft}^3}{EI} \downarrow$$

Ans.

Ans.

15 k

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B ____

-15 ft-

30 ft -

8–13. Solve Prob. 8–12 using the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c,

$$\zeta + \sum M_A = 0; \quad B'_y(30 \text{ ft}) - \left[\frac{1}{2} \left(\frac{225 \text{ k} \cdot \text{ft}}{EI}\right)(30 \text{ ft})\right](20 \text{ ft})$$
$$B'_y = \frac{2250 \text{ k} \cdot \text{ft}^2}{EI}$$

Referring to Fig. d,

$$+\uparrow \sum F_{y} = 0; \quad -V_{C}' - \frac{1}{2} \left(\frac{225 \text{ k} \cdot \text{ft}}{EI}\right) (15 \text{ ft}) - \frac{2250 \text{ k} \cdot \text{ft}}{EI}$$
$$\theta_{C} = V_{C}' = -\frac{3937.5 \text{ k} \cdot \text{ft}^{2}}{EI} = \frac{3937.5 \text{ k} \cdot \text{ft}^{2}}{EI} \quad \forall \qquad \text{Ans.}$$

$$\zeta + \sum M_C = 0; \quad M'_C + \left\lfloor \frac{1}{2} \left(\frac{223 \text{ k} \cdot \text{ft}}{EI} \right) (15 \text{ ft}) \right\rfloor (10 \text{ ft}) + \left(\frac{2230 \text{ k} \cdot \text{ft}}{EI} \right) (15 \text{ ft})$$
$$\Delta_C = M'_C = \frac{50625 \text{ k} \cdot \text{ft}^3}{EI} = \frac{50625 \text{ k} \cdot \text{ft}^3}{EI} \quad \downarrow \qquad \text{Ans.}$$

8–14. Determine the value of *a* so that the slope at *A* is equal to zero. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\begin{aligned} \theta_{A/B} &= \frac{1}{2} \left(\frac{PL}{4EI} \right) (L) + \frac{1}{2} \left(-\frac{Pa}{EI} \right) (a+L) \\ &= \frac{PL^2}{8EI} - \frac{Pa^2}{2EI} - \frac{PaL}{2EI} \\ t_{D/B} &= \left[\frac{1}{2} \left(\frac{PL}{4EI} \right) (L) \right] \left(\frac{L}{2} \right) + \left[\frac{1}{2} \left(-\frac{Pa}{EI} \right) (L) \right] \left(\frac{L}{3} \right) \\ &= \frac{PL^3}{16EI} - \frac{PaL^2}{6EI} \end{aligned}$$

Then

$$\theta_B = \frac{t_{D/B}}{L} = \frac{PL^2}{16EI} - \frac{PaL}{6EI}$$

Here, it is required that

$$\theta_B = \theta_{A/B}$$

$$\frac{PL^2}{16EI} - \frac{PaL}{6EI} = \frac{PL^2}{8EI} - \frac{Pa^2}{2EI} - \frac{PaL}{2EI}$$

$$24a^2 + 16La - 3L^2 = 0$$

Choose the position root,

$$a = 0.153 L$$

Ans.

8–15. Solve Prob. 8–14 using the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. d,

$$\begin{aligned} \zeta + \sum M_B &= 0; \quad D'_y(L) + \left[\frac{1}{2}\left(\frac{Pa}{EI}\right)(L)\right]\left(\frac{2}{3}L\right) - \left[\frac{1}{2}\left(\frac{PL}{4EI}\right)(L)\right]\left(\frac{1}{2}\right) &= 0\\ D'_y &= \frac{PL^2}{16EI} - \frac{PaL}{3EI} \end{aligned}$$

It is required that $V'_A = \theta_A = 0$, Referring to Fig. c,

$$\uparrow + \sum F_y = 0; \quad \frac{PL^2}{16EI} - \frac{PaL}{3EI} - \frac{Pa^2}{2EI} = 0$$
$$24a^2 + 16La - 3L^2 = 0$$

Choose the position root,

$$a = 0.153 L$$

***8–16.** Determine the value of a so that the displacement at C is equal to zero. EI is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 2 gives

$$\begin{split} t_{D/B} &= \left[\frac{1}{2} \left(\frac{PL}{4EI}\right)(L)\right] \left(\frac{L}{2}\right) + \left[\frac{1}{2} \left(-\frac{Pa}{EI}\right)(L)\right] \left(\frac{L}{3}\right) \\ &= \frac{PL^3}{16EI} - \frac{PaL^2}{6EI} \\ T_{C/B} &= \left[\frac{1}{2} \left(\frac{PL}{4EI}\right) \left(\frac{L}{2}\right)\right] \left[\frac{1}{3} \left(\frac{L}{2}\right)\right] + \left[\frac{1}{2} \left(-\frac{Pa}{2EI}\right) \left(\frac{L}{2}\right)\right] \left[\frac{1}{3} \left(\frac{L}{2}\right)\right] \\ &= \frac{PL^3}{96EI} - \frac{PaL^2}{48EI} \end{split}$$

It is required that

tanc

8–17. Solve Prob. 8–16 using the conjugate-beam method. A • D The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c, ρ $\left[\frac{1}{2}\left(\frac{PL}{4EI}\right)(L)\right]\left(\frac{L}{2}\right) - \left[\frac{1}{2}\left(\frac{Pa}{EI}\right)(L)\right]\left(\frac{L}{3}\right) - B'_{y}(L) = 0$ $\zeta + \sum M_D = 0;$ $-B'_y = \frac{PL^2}{16EI} - \frac{PaL}{6EI}$ Here, it is required that $M'_C = \Delta_C = 0$. Referring to Fig. d, $\zeta + \sum M_C = 0; \qquad \left[\frac{1}{2} \left(\frac{PL}{4EI}\right) \left(\frac{L}{2}\right)\right] \left[\frac{1}{3} \left(\frac{L}{2}\right)\right]$ $\frac{p}{2L}$ (2a+3L) $\frac{1}{2L}(L-za)$ $-\left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)\left(\frac{L}{2}\right)\right]\left[\frac{1}{3}\left(\frac{L}{2}\right)\right]$ MEI $-\left[\frac{PL^2}{16EI} - \frac{PaL}{6EI}\right]\left(\frac{L}{2}\right) = 0$ <u>PL</u> 4EI $\frac{PL^3}{96EI} - \frac{PaL^2}{48EI} - \frac{PL^3}{32EI} + \frac{PaL^2}{12EI} = 0$ a+L $\frac{L}{96} - \frac{a}{48} - \frac{L}{32} + \frac{a}{12} = 0$ a+L <u>-ра</u> ЕІ $a = \frac{L}{3}$ Ans. (a) 」 <u>
-</u>(<u>PL</u>)(<u>L</u>) (^上)(^上) a (生) EI M_c=0 GAUS $\frac{L}{3}$ 출L By' Dy $\frac{1}{2} \left(\frac{\rho_a}{z_{EI}} \right) \left(\frac{L}{z} \right) \frac{B_{\gamma}}{z}$ Pa ET (d)(b) (C)

8-18. Determine the slope and the displacement at C. EI is constant. Use the moment-area theorems. B _____ Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give $t_{B/D} = \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(a)\right]\left(\frac{2}{3}a\right) = \frac{Pa^3}{6EI}$ $t_{C/D} = \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(a)\right]\left(a + \frac{2}{3}a\right) = \frac{5Pa^3}{12EI}$ $\theta_{C/D} = \frac{1}{2} \left(\frac{Pa}{2EI} \right) (a) = \frac{Pa^2}{4EI}$ Then, $\theta_C = \theta_{C/D} = \frac{Pa^2}{4EI}$ Ans. $\Delta_C = t_{C/D} - t_{B/D} = \frac{5Pa^3}{12EI} - \frac{Pa^3}{6EI} = \frac{Pa^3}{4EI} \quad \uparrow$ Ans. P a a a

8–19. Solve Prob. 8–18 using the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c,

$$\zeta + \sum M_A = 0;$$
 $\left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(2a)\right](a) - B'_y(2a) = 0 \quad B'_y = \frac{Pa^2}{4EI}$

P

Referring to Fig. d

$$+\uparrow \sum F_y = 0;$$
 $\frac{Pa^2}{4EI} - V'_C = 0$ $\theta_C = V'_C = \frac{Pa^2}{4EI}$ Ans.

$$\zeta + \sum M_C = 0; \quad M'_C - \frac{Pa^2}{4EI}(a) = 0 \quad \Delta_C = M'_C = \frac{Pa^3}{4EI} \uparrow$$
 Ans.

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*8–20. Determine the slope and the displacement at the end C of the beam. E = 200 GPa, $I = 70(10^6)$ mm⁴. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\begin{aligned} \theta_{C/A} &= \frac{1}{2} \left(\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (6 \text{ m}) + \frac{1}{2} \left(-\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (9 \text{ m}) \\ &= -\frac{18 \text{ kN} \cdot \text{m}}{EI} = \frac{18 \text{ kN} \cdot \text{m}}{EI} \quad \bigtriangledown \\ t_{B/A} &= \left[\frac{1}{2} \left(\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (6 \text{ m}) \right] (3 \text{ m}) + \left[\frac{1}{2} \left(-\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (6 \text{ m}) \right] \left[\frac{1}{3} (6 \text{ m}) \right] \\ &= \frac{36 \text{ kN} \cdot \text{m}^3}{EI} \\ t_{C/A} &= \left[\frac{1}{2} \left(\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (6 \text{ m}) \right] (6 \text{ m}) + \left[\frac{1}{2} \left(-\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (6 \text{ m}) \right] \left[3 \text{ m} + \frac{1}{3} (6 \text{ m}) \\ &+ \left[\frac{1}{2} \left(-\frac{12 \text{ kN} \cdot \text{m}}{EI} \right) (3 \text{ m}) \right] \left[\frac{2}{3} (3 \text{ m}) \right] \\ &= 0 \end{aligned}$$

Then

 $= 3.86 \text{ mm} \downarrow \text{Ans.}$

8 kN

D

- 3 m -

8–21. Solve Prob. 8–20 using the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c

$$\zeta + \sum M_A = 0; \quad B'_y(6 \text{ m}) + \left[\frac{1}{2}\left(\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(6 \text{ m})\right](3 \text{ m})$$
$$- \left[\frac{1}{2}\left(\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(6 \text{ m})\right]\left[\frac{2}{3}(6 \text{ m})\right] = 0$$
$$B'_y = \frac{6 \text{ kN} \cdot \text{m}^2}{EI}$$

Referring to Fig. d,

$$\begin{aligned} \zeta + \sum Fy &= 0; \quad -V'_C - \frac{6 \,\mathrm{kN} \cdot \mathrm{m}^2}{EI} - \frac{1}{2} \left(\frac{12 \,\mathrm{kN} \cdot \mathrm{m}}{EI} \right) (3 \,\mathrm{m}) \,=\, 0 \\ \theta_C &= V'_C \,=\, -\frac{24 \,\mathrm{kN} \cdot \mathrm{m}^2}{EI} \,=\, \frac{24(10^3) \,\mathrm{N} \cdot \mathrm{m}^2}{\left[(200(10^9) \,\mathrm{N}/\mathrm{m}^2) \right] \left[(70(10^{-6}) \,\mathrm{m}^4 \right] \right]} \\ &= 0.00171 \,\mathrm{rad} \quad \bigtriangledown \qquad \mathbf{Ans.} \end{aligned}$$
$$\begin{aligned} \zeta + \sum M_C &= 0; \quad M'_C \,+\, \left[\frac{1}{2} \left(\frac{12 \,\mathrm{kN} \cdot \mathrm{m}^2}{EI} \right) (3 \,\mathrm{m}) \right] \left[\frac{2}{3} (3 \,\mathrm{m}) \right] \\ &+\, \left(\frac{6 \,\mathrm{kN} \cdot \mathrm{m}^2}{EI} \right) (3 \,\mathrm{m}) \,=\, 0 \end{aligned}$$

$$\Delta_C = M'_C = -\frac{54 \text{ kN} \cdot \text{m}^3}{EI} = \frac{54 (10^3) \text{ N} \cdot \text{m}^3}{[200(10^9)\text{N/m}^2] [70(10^{-6})\text{m}^4]}$$

= 0.00386 m = 3.86 mm \U03c6 Ans.

4 kN

8–22. At what distance a should the bearing supports at A and B be placed so that the displacement at the center of the shaft is equal to the deflection at its ends? The bearings exert only vertical reactions on the shaft. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively. Theorem 2 gives

 $t_{B/C} = \left(-\frac{Pa}{EI}\right) \left(\frac{L-2a}{2}\right) \left(\frac{L-2a}{4}\right) = -\frac{Pa}{8EI} (L-2a)^2$ $t_{D/C} = \left(-\frac{Pa}{EI}\right) \left(\frac{L-2a}{2}\right) \left(a + \frac{L-2a}{4}\right) + \frac{1}{2} \left(-\frac{Pa}{EI}\right) (a) \left(\frac{2}{3}a\right)$ $= -\left[\frac{Pa}{8EI} (L^2 - 4a^2) + \frac{Pa^3}{3EI}\right]$

It is required that

$$t_{D/C} = 2 t_{B/C}$$

$$\frac{Pa}{8EI}(L^2 - 4a^2) + \frac{Pa^3}{3EI} = 2\left[\frac{Pa}{8EI} - (L - 2a)^2\right]$$

$$\frac{7Pa^3}{6EI} - \frac{Pa^2L}{EI} + \frac{PaL^2}{8EI} = 0$$

$$56a^2 - 48La + 6L^2 = 0$$

Choose

a = 0.152 L

8–23. Solve Prob. 8–22 using the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. *a* and *b*, respectively. Referring to Fig. c,

$$\begin{aligned} \zeta + \sum M_A &= 0; \quad B'_y \left(L - 2a \right) - \left[\frac{Pa}{EI} \left(L - 2a \right) \right] \left(\frac{L - 2a}{2} \right) &= 0 \\ B'_y &= \frac{Pa}{2EI} (L - 2a) \end{aligned}$$

Referring to Fig. d,

$$M'_{D} + \frac{Pa}{2EI}(L - 2a)(a) + \left[\frac{1}{2}\left(\frac{Pa}{EI}\right)(a)\right]\left(\frac{2}{3}a\right) = 0$$

$$\Delta_{D} = M'_{D} = -\left[\frac{Pa^{2}}{2EI}(L - 2a) + \frac{Pa^{3}}{3EI}\right]$$

Referring to Fig. e,

$$\frac{Pa}{2EI}(L-2a)\left(\frac{L-2a}{2}\right) - \frac{Pa}{EI}\left(\frac{L-2a}{2}\right)\left(\frac{L-2a}{4}\right) - M'_{C} = 0$$
$$\Delta_{C} = M'_{C} = \frac{Pa}{8EI}(L-2a)^{2}$$

It is required that

$$\begin{aligned} |\Delta_D| &= \Delta_C \\ \frac{Pa^2}{2EI}(L-2a) + \frac{Pa^3}{3EI} &= \frac{Pa}{8EI}(L-2a)^2 \\ \frac{7Pa^3}{6EI} - \frac{Pa^2L}{EI} + \frac{PaL^2}{8EI} &= 0 \\ 56a^2 - 48La + 6L^2 &= 0 \end{aligned}$$

Choose

$$a = 0.152 L$$

***8–24.** Determine the displacement at C and the slope at B. EI is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\theta_{B/D} = \left(-\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(1.5 \text{ m}) = -\frac{18 \text{ kN} \cdot \text{m}^2}{EI}$$

$$t_{B/D} = \left[\left(-\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(1.5 \text{ m})\right] \left[\frac{1}{2}(1.5 \text{ m})\right] = \frac{13.5 \text{ kN} \cdot \text{m}^3}{EI}$$

$$t_{C/D} = \left[\left(-\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(1.5 \text{ m})\right] \left[\frac{1}{2}(1.5 \text{ m}) + 3 \text{ m}\right] + \left[\frac{1}{2}\left(-\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(3 \text{ m})\right] \left[\frac{2}{3}(3 \text{ m})\right]$$

$$= \frac{103.5 \text{ kN} \cdot \text{m}^3}{EI}$$

Then,

The real beam and conjugate beam are shown in Fig. *a* and *b*, respectively. Referring to Fig. *c*,

$$\zeta + \sum M_A = 0; \qquad B'_y (3 \text{ m}) - \left(\frac{12 \text{ kN} \cdot \text{m}}{EI}\right) (3 \text{ m})(1.5 \text{ m}) = 0$$
$$B'_y = \theta_B = \frac{18 \text{ kN} \cdot \text{m}^2}{EI} \quad \forall \qquad \text{Ans.}$$

Referring to Fig.d,

$$\zeta + \sum M_C = 0; \quad M'_C + \left(\frac{18 \text{ kN} \cdot \text{m}^2}{EI}\right)(3 \text{ m}) + \left[\frac{1}{2}\left(\frac{12 \text{ kN} \cdot \text{m}}{EI}\right)(3 \text{ m})\right] \left[\frac{2}{3}(3 \text{ m})\right] = 0$$
$$\Delta_C = M'_C = -\frac{90 \text{ kN} \cdot \text{m}^3}{EI} = \frac{90 \text{ kN} \cdot \text{m}^3}{EI} \downarrow \qquad \text{Ans.}$$

8–26. Determine the displacement at *C* and the slope at *B*. *EI* is constant. Use the moment-area theorems.

 $\begin{array}{c} \mathbf{P} \\ \mathbf{$

Using the $\frac{M}{EI}$ diagram and elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\begin{aligned} \theta_{B/C} &= \frac{1}{2} \left(\frac{Pa}{EI} \right) (a) + \left(\frac{Pa}{EI} \right) (a) + \frac{1}{2} \left(\frac{Pa}{2EI} \right) (a) = \frac{7Pa^2}{4EI} \\ t_{B/C} &= \left[\frac{1}{2} \left(\frac{Pa}{EI} \right) (a) \right] \left(\frac{2}{3}a \right) + \left[\frac{Pa}{EI} (a) \right] \left(a + \frac{1}{2}a \right) + \left[\frac{1}{2} \left(\frac{Pa}{2EI} \right) (a) \right] \left(a + \frac{2}{3}a \right) \\ &= \frac{9Pa^3}{4EI} \end{aligned}$$

Then

8–27. Determine the displacement at *C* and the slope at *B*. *EI* is constant. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c,

$$\zeta + \sum M_A = 0; \qquad \left[\frac{1}{2}\left(\frac{Pa}{EI}\right)(a)\right]\left(\frac{2}{3}a\right) + \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(a)\right]\left(\frac{10}{3}a\right) \\ + \left[\left(\frac{Pa}{EI}\right)(2a) + \frac{1}{2}\left(\frac{Pa}{2EI}\right)(2a)\right](2a) \\ -B'_y = (4a) = 0$$

$$\theta_B = B'_y = \frac{7Pa^2}{4EI} \quad \bigtriangledown$$

Referring to Fig. d,

$$\zeta + \sum M_C = 0; \qquad \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(a)\right]\left(\frac{4}{3}a\right) + \left[\left(\frac{Pa}{EI}\right)(a)\right]\left(\frac{a}{2}\right) \\ + \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(a)\right]\left(\frac{a}{3}\right) - \frac{7Pa^2}{4EI}(2a) \\ -M_C' = 0 \\ \Delta_C = M_C' = -\frac{9Pa^3}{4EI} = \frac{9Pa^3}{4EI} \quad \downarrow$$

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***8–28.** Determine the force \mathbf{F} at the end of the beam *C* so that the displacement at *C* is zero. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and elastic curve shown in Fig. *a* and *b*, respectively, Theorem 2 gives

$$t_{B/A} = \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(2a)\right](a) + \left[\frac{1}{2}\left(-\frac{Fa}{EI}\right)(2a)\right]\left[\frac{1}{3}(2a)\right] = \frac{Pa^3}{2EI} - \frac{2Fa^3}{3EI}$$

$$t_{C/A} = \left[\frac{1}{2}\left(\frac{Pa}{2EI}\right)(2a)\right](2a) + \left[\frac{1}{2}\left(-\frac{Fa}{EI}\right)(2a)\right]\left[\frac{1}{3}(2a) + a\right]$$

$$+ \left[\frac{1}{2}\left(-\frac{Fa}{EI}\right)(a)\right]\left[\frac{2}{3}(a)\right]$$

$$= \frac{Pa^3}{2EI} - \frac{2Fa^3}{3EI}$$

It is required that

$$t_{C/A} = \frac{3}{2} t_{B/A}$$

$$\frac{Pa^3}{EI} - \frac{2Fa^3}{EI} = \frac{3}{2} \left[\frac{Pa^3}{2EI} - \frac{2Fa^3}{3EI} \right]$$

$$F = \frac{P}{4}$$

8–29. Determine the force **F** at the end of the beam C so that the displacement at C is zero. EI is constant. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. *a* and *b*, respectively. Referring to Fig. *c*,

$$\begin{aligned} \zeta + \sum M_A &= 0; \quad \left[\frac{1}{2} \left(\frac{Pa}{2EI}\right)(2a)\right](a) - \left[\frac{1}{2} \left(\frac{Fa}{EI}\right)(2a)\right]\left[\frac{2}{3}\left(2a\right)\right] - B'_y(2a) = 0\\ B'_y &= \frac{Pa^2}{4EI} - \frac{2Fa^2}{3EI} \end{aligned}$$

Here, it is required that $\Delta_C = M'_C = 0$. Referring to Fig. d,

Ans.

D

B

8–30. Determine the slope at *B* and the displacement at *C*. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and elastic curve shown in Fig. *a* and *b*, Theorem 1 and 2 give

$$\theta_{B/A} = \frac{1}{2} \left(-\frac{Pa}{EI} \right) (a) = -\frac{Pa^2}{2EI} = \frac{Pa^2}{2EI} \quad \forall$$
$$t_{B/A} = \left[\frac{1}{2} \left(-\frac{Pa}{EI} \right) (a) \right] \left[\frac{1}{3} (a) \right] = -\frac{Pa^3}{6EI}$$
$$t_{C/A} = \left[\frac{1}{2} \left(-\frac{Pa}{EI} \right) (2a) \right] (a) = -\frac{Pa^3}{EI}$$

Then

$$\theta_{A} = \frac{|t_{B/A}|}{L_{AB}} = \frac{Pa^{3}/6EI}{2a} = \frac{Pa^{2}}{12EI} \succeq$$
$$\Delta' = \frac{3}{2} |t_{B/A}| = \frac{3}{2} \left(\frac{Pa^{3}}{6EI}\right) = \frac{Pa^{3}}{4EI}$$
$$\theta_{B} = \theta_{A} + \theta_{B/A}$$
$$\zeta + \theta_{B} = -\frac{Pa^{2}}{12EI} + \frac{Pa^{2}}{2EI} = \frac{5Pa^{2}}{12EI} \checkmark$$
$$\Delta_{C} = |t_{C/A}| - \Delta' = \frac{Pa^{3}}{EI} - \frac{Pa^{3}}{4EI} = \frac{3Pa^{3}}{4EI} \downarrow$$

Ans.

Ans.

С

B____

8–31. Determine the slope at *B* and the displacement at *C*. *EI* is constant. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. c and d, respectively. Referring to Fig. d,

Referring to Fig. c,

$$\zeta + \sum M_C = 0; \quad -M'_C - \left[\frac{1}{2}\left(\frac{Pa}{EI}\right)(a)\right]\left(\frac{2}{3}a\right) - \left(\frac{5Pa^2}{12EI}\right)(a) = 0$$
$$\Delta_C = M'_C = -\frac{3Pa^3}{4EI} = \frac{3Pa^3}{4EI} \quad \downarrow \qquad \text{Ans.}$$

***8–32.** Determine the maximum displacement and the slope at *A*. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\theta_{D/A} = \frac{1}{2} \left(\frac{M_0}{EIL} x \right) (x) = \frac{M_0}{2EIL} x^2 \quad \measuredangle$$
$$t_{B/A} = \left[\frac{1}{2} \left(\frac{M_0}{2EI} \right) \left(\frac{L}{2} \right) \right] \left[\frac{1}{3} \left(\frac{L}{2} \right) \right] = \frac{M_0 L^2}{48EI}$$

Then,

$$\theta_A = \frac{|t_{B/A}|}{L_{AB}} = \frac{M_0 L^2 / 48 EI}{L/2} = \frac{M_0 L}{24 EI}$$
 \bigtriangledown

Here $\theta_D = 0$. Thus,

$$\begin{aligned} \theta_D &= \theta_A + \theta_{D/A} \\ \zeta + & 0 = -\frac{M_0 L}{24EI} + \frac{M_0}{2EIL} x^2 \qquad x = \frac{L}{\sqrt{12}} = 0.2887L \\ \Delta_{\max} &= \Delta_D = t_{B/D} = \left[\frac{1}{2} \left(\frac{0.2113M_0}{EI}\right) (0.2113L)\right] \left[\frac{1}{3} (0.2113L)\right] \\ & + \left[\left(\frac{0.2887M_0}{EI}\right) (0.2113L)\right] \left[\frac{1}{2} (0.2113L)\right] \\ &= \frac{0.00802M_0 L^2}{EI} \quad \downarrow \end{aligned}$$

Ans.

Ans.

8–33. Determine the maximum displacement at *B* and the slope at *A*. *EI* is constant. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c

$$\begin{aligned} \zeta + \sum M_B &= 0; \quad A'_y(L) - \left[\frac{1}{2} \left(\frac{M_0}{2EI}\right) \left(\frac{L}{2}\right)\right] \left(\frac{L}{3}\right) = 0\\ A'_y &= \theta_A = \frac{M_0 L}{24EI} \end{aligned}$$

Here it is required that $\theta_D = V'_D = 0$. Referring to Fig. d,

$$\uparrow \sum F_y = 0; \quad \frac{1}{2} \left(\frac{M_0}{EIL} x \right) (x) - \frac{M_0 L}{24EI} = 0$$

$$x = \frac{L}{\sqrt{12}}$$

$$\zeta + \sum M_D = 0; \quad M'_D + \left(\frac{M_0 L}{24EI} \right) \left(\frac{L}{\sqrt{12}} \right)$$

$$- \frac{1}{2} \left(\frac{M_0}{EIL} \right) \left(\frac{L}{\sqrt{12}} \right) \left(\frac{L}{\sqrt{12}} \right) \left[\frac{1}{3} \left(\frac{L}{\sqrt{12}} \right) \right] = 0$$

$$\Delta_{\text{max}} = \Delta_D = M'_D = -\frac{0.00802M_0 L^2}{EI}$$

$$= \frac{0.00802M_0 L^2}{EI} \quad \downarrow$$

(a)

Ans.

Α

 $M_0 = Pa$

B

C

8–34. Determine the slope and displacement at *C*. *EI* is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\theta_{C} = |\theta_{C/A}| = \frac{1}{2} \left(\frac{Pa}{EI}\right)(a) + \left(\frac{2Pa}{EI}\right)(a) + \frac{1}{2} \left(\frac{Pa}{EI}\right)(a) = \frac{3Pa^{2}}{EI} \quad \forall \qquad \text{Ans}$$

$$\Delta_{C} = |t_{C/A}| = \left[\frac{1}{2} \left(\frac{Pa}{EI}\right)(a)\right] \left(a + \frac{2}{3}a\right) + \left[\left(\frac{2Pa}{EI}\right)(a)\right] \left(a + \frac{a}{2}\right) + \left[\frac{1}{2} \left(\frac{Pa}{EI}\right)(a)\right] \left(\frac{2}{3}a\right) = 0$$

$$= \frac{25Pa^{3}}{6EI} \quad \downarrow \qquad \text{Ans}$$

D

A

8–35. Determine the slope and displacement at *C*. *EI* is constant. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c,

$$+\uparrow \sum F_{y} = 0; \quad -V_{C}' - \frac{1}{2} \left(\frac{Pa}{EI}\right)(a) - \left(\frac{2Pa}{EI}\right)(a) - \frac{1}{2} \left(\frac{Pa}{EI}\right)(a) = 0$$
$$\theta_{C} = V_{C}' = -\frac{3Pa^{2}}{EI} = \frac{3Pa^{2}}{EI} \quad \forall \qquad \text{Ans.}$$
$$\zeta + \sum M_{C} = 0; \quad M_{C}' + \left[\frac{1}{2} \left(\frac{Pa}{EI}\right)(a)\right] \left(\frac{2}{3}a\right) + \left[\left(\frac{2Pa}{EI}\right)(a)\right] \left(a + \frac{a}{2}\right)$$
$$+ \left[\frac{1}{2} \left(\frac{Pa}{EI}\right)(a)\right] \left(a + \frac{2}{3}a\right) = 0$$
$$\Delta_{C} = M_{C}' = -\frac{25Pa^{3}}{6EI} = \frac{25Pa^{3}}{6EI} \quad \downarrow \qquad \text{Ans.}$$

 $\begin{array}{c} P \\ 3Pa \\ 3Pa \\ \hline a \\ \hline$

р

С

 $M_0 = Pa$

B

***8–36.** Determine the displacement at C. Assume A is a fixed support, B is a pin, and D is a roller. EI is constant. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and the elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\Delta_B = |t_{B/A}| = \left[\frac{1}{2}\left(\frac{37.5 \text{ kN} \cdot \text{m}}{EI}\right)(3 \text{ m})\right] \left[\frac{2}{3}(3 \text{ m})\right] = \frac{112.5 \text{ kN} \cdot \text{m}^3}{EI} \downarrow$$
$$t_{C/D} = \left[\frac{1}{2}\left(\frac{37.5 \text{ kN} \cdot \text{m}}{EI}\right)(3 \text{ m})\right] \left[\frac{1}{3}(3 \text{ m})\right] = \frac{56.25 \text{ kN} \cdot \text{m}^3}{EI}$$
$$t_{B/D} = \left[\frac{1}{2}\left(\frac{37.5 \text{ kN} \cdot \text{m}}{EI}\right)(6 \text{ m})\right](3 \text{ m}) = \frac{337.5 \text{ kN} \cdot \text{m}^3}{EI}$$

Then

$$\theta_{D} = \frac{\Delta_{B} + t_{B/D}}{L_{B/D}} = \frac{112.5 \text{ kN} \cdot \text{m}^{3}/EI + 337.5 \text{ kN} \cdot \text{m}^{3}/EI}{6 \text{ m}} = \frac{75 \text{ kN} \cdot \text{m}^{2}}{EI} \quad \forall \quad \text{Ans.}$$

$$\Delta_{C} + t_{C/D} = \frac{1}{2} (\Delta_{B} + t_{B/D})$$

$$\Delta_{C} + \frac{56.25 \text{ kN} \cdot \text{m}^{3}}{EI} = \frac{1}{2} \left(\frac{112.5 \text{ kN} \cdot \text{m}^{3}}{EI} + \frac{337.5 \text{ kN} \cdot \text{m}^{3}}{EI} \right)$$

$$\Delta_{C} = \frac{169 \text{ kN} \cdot \text{m}^{3}}{EI} \downarrow \qquad \text{Ans.}$$

8–37. Determine the displacement at C. Assume A is a fixed support, B is a pin, and D is a roller. EI is constant. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c,

$$\zeta + \sum M_B = 0; \quad \left[\frac{1}{2} \left(\frac{37.5 \text{ kN} \cdot \text{m}}{EI}\right) (6 \text{ m})\right] (3 \text{ m}) \\ + \left[\frac{1}{2} \left(\frac{37.5 \text{ kN} \cdot \text{m}}{EI}\right) (3 \text{ m})\right] (2 \text{ m}) - D'_y (6 \text{ m}) = 0 \\ \theta_D = D'_y = \frac{75 \text{ kN} \cdot \text{m}^2}{EI} \overleftarrow{\Sigma}$$

Referring to Fig. d,

$$\zeta + \sum M_C = 0; \quad \left[\frac{1}{2} \left(\frac{37.5 \text{ kN} \cdot \text{m}}{EI}\right) (3 \text{ m})\right] (1 \text{ m})$$
$$-\left(\frac{75 \text{ kN} \cdot \text{m}^2}{EI}\right) (3 \text{ m}) - M'_C = 0$$
$$\Delta_C = -\frac{168.75 \text{ kN} \cdot \text{m}^3}{EI} = \frac{168.75 \text{ kN} \cdot \text{m}^3}{EI} \downarrow$$

(d)

25 kN

D

3 m

0

3 m

B

-3 m

8–38. Determine the displacement at D and the slope at D. Assume A is a fixed support, B is a pin, and C is a roller. Use the moment-area theorems.

Using the $\frac{M}{EI}$ diagram and elastic curve shown in Fig. *a* and *b*, respectively, Theorem 1 and 2 give

$$\Delta_B = t_{B/A} = \left[\frac{1}{2}\left(\frac{72 \text{ k} \cdot \text{ft}}{EI}\right)(12 \text{ ft})\right] \left[\frac{2}{3}(12 \text{ ft})\right] = \frac{3456 \text{ k} \cdot \text{ft}^3}{EI} \uparrow$$

$$\theta_{D/B} = \frac{1}{2}\left(-\frac{72 \text{ k} \cdot \text{ft}}{EI}\right)(24 \text{ ft}) = -\frac{864 \text{ k} \cdot \text{ft}^2}{EI} = \frac{864 \text{ k} \cdot \text{ft}^2}{EI} \bigtriangledown$$

$$t_{C/B} = \left[\frac{1}{2}\left(-\frac{72 \text{ k} \cdot \text{ft}}{EI}\right)(12 \text{ ft})\right] \left[\frac{1}{3}(12 \text{ ft})\right] = -\frac{1728 \text{ k} \cdot \text{ft}^3}{EI}$$

$$t_{D/B} = \left[\frac{1}{2}\left(-\frac{72 \text{ k} \cdot \text{ft}}{EI}\right)(24 \text{ ft})\right](12 \text{ ft}) = -\frac{10368 \text{ k} \cdot \text{ft}^3}{EI}$$

Then,

1

$$\Delta' = 2(\Delta_B - |t_{C/B}| = 2\left(\frac{3456 \text{ k} \cdot \text{ft}^3}{EI} - \frac{1728 \text{ k} \cdot \text{ft}^3}{EI}\right) = \frac{3456 \text{ k} \cdot \text{ft}^3}{EI}$$

$$\theta_{BR} = \frac{\Delta'}{L_{BD}} = \frac{3456 \text{ k} \cdot \text{ft}^3/EI}{24 \text{ ft}} = \frac{144 \text{ k} \cdot \text{ft}^2}{EI} \nabla$$

$$\theta_D = \theta_{BR} + \theta_{D/B}$$

$$+ \mathcal{O} \theta_D = \frac{144 \text{ k} \cdot \text{ft}^2}{EI} + \frac{864 \text{ k} \cdot \text{ft}^2}{EI} = \frac{1008 \text{ k} \cdot \text{ft}^2}{EI} \nabla$$

$$\Delta_D = |t_{D/B}| + \Delta' - \Delta_B$$

$$= \frac{10368 \text{ k} \cdot \text{ft}^3}{EI} + \frac{3456 \text{ k} \cdot \text{ft}^3}{EI} - \frac{3456 \text{ k} \cdot \text{ft}^3}{EI}$$

$$= \frac{10,368 \text{ k} \cdot \text{ft}^3}{EI} \downarrow$$

$$\int \frac{72 \text{ k} \text{ ft}}{EI} \int \frac{6 \text{ k}}{EI} \int \frac{12 \text{ k} \text{ ft}}{EI} \int \frac{24 \text{ k} \text{ s}}{EI} \int \frac{24 \text{ s$$

Ans.

Ans.

ΕI

(a)

8–39. Determine the displacement at D and the slope at D. Assume A is a fixed support, B is a pin, and C is a roller. Use the conjugate-beam method.

The real beam and conjugate beam are shown in Fig. a and b, respectively. Referring to Fig. c,

$$\zeta + \sum M_B = 0; \qquad C'_y(12 \text{ ft}) - \left[\frac{1}{2} \left(\frac{72 \text{ k} \cdot \text{ft}}{EI}\right)(12 \text{ ft})\right](16 \text{ ft})$$
$$= 0$$
$$C'_y = \frac{576 \text{ k} \cdot \text{ft}^2}{EI}$$

Referring to Fig. d,

+ $\sum F_y = 0; \quad -V'_D - \frac{1}{2} \left(\frac{72 \text{ k} \cdot \text{ft}}{EI} \right) (12 \text{ ft}) - \frac{576 \text{ k} \cdot \text{ft}^2}{EI} = 0$

$$\theta_D = V'_D = -\frac{1008 \operatorname{k} \cdot \operatorname{ft}^2}{EI} = \frac{1008 \operatorname{k} \cdot \operatorname{ft}^2}{EI} \quad \forall \quad \text{Ans.}$$

$$\zeta + \sum M_C = 0; \quad M'_D + \left[\frac{1}{2}\left(\frac{72 \text{ k} \cdot \text{ft}}{EI}\right)(12 \text{ ft})\right](8 \text{ ft}) + \left(\frac{576 \text{ k} \cdot \text{ft}^2}{EI}\right)(12 \text{ ft}) = 0$$

$$10368 \text{ k} \cdot \text{ft}^3 = 10.368 \text{ k} \cdot \text{ft}^3$$

$$M'_D = \Delta_D = -\frac{10368 \text{ k} \cdot \text{ft}^3}{EI} = \frac{10,368 \text{ k} \cdot \text{ft}^3}{EI} \downarrow$$
 Ans.

